

**Spherical Symmetry**  $\rightarrow$  all field quantities are functions of  $r$  only, i.e. no  $\theta$  or  $\phi$  dependence.

Furthermore, since the stress field at a given point is invariant under any rotation of the coordinate axes it can be argued that  $\sigma_{r\theta} = \sigma_{r\phi} = \sigma_{\theta\phi} = 0$  and  $\sigma_{\theta\theta} = \sigma_{\phi\phi}$  by rotating the coordinate system by  $90^\circ$  about the  $r$ -direction.

Compatibility:  $\epsilon_{rr} = \frac{du_r}{dr}$ ,  $\epsilon_{\theta\theta} = \frac{u_r}{r}$

Hooke's Law :  $\epsilon_{rr} = \frac{1}{E} \sigma_{rr} - \frac{\nu}{E} \sigma_{\theta\theta}$  (3)  
 $\epsilon_{\theta\theta} = \frac{1-\nu}{E} \sigma_{\theta\theta} - \frac{\nu}{E} \sigma_{rr}$  (4)

$$\epsilon_{\theta\theta} = \frac{1-\nu}{E} \sigma_{\theta\theta} - \frac{\nu}{E} \sigma_{rr} \quad (4)$$

4 Equations for 4 unknowns ( $\sigma_{rr}, \sigma_{\theta\theta}, \epsilon_{rr}, \epsilon_{\theta\theta}$ )

$$\textcircled{1} \rightarrow \sigma_{\theta\theta} = \frac{r}{2} \frac{d\sigma_{rr}}{dr} + \sigma_{rr}$$

$$\textcircled{3} \rightarrow \varepsilon_{rr} = \frac{1}{E} \sigma_{rr} - \frac{2\nu}{E} \sigma_{rr} - \frac{\nu}{E} r \frac{d\sigma_{rr}}{dr}$$

$$\varepsilon_{rr} = \frac{1-2\nu}{E} \sigma_{rr} - \frac{\nu}{E} r \frac{d\sigma_{rr}}{dr}$$

$$\textcircled{4} \rightarrow \varepsilon_{\theta\theta} = \frac{1-\nu}{E} \sigma_{rr} + \frac{1+\nu}{E} \frac{r}{2} \frac{d\sigma_{rr}}{dr} - \frac{\nu}{E} \sigma_{rr}$$

$$\varepsilon_{\theta\theta} = \frac{1-2\nu}{E} \sigma_{rr} + \frac{1+\nu}{2E} r \frac{d\sigma_{rr}}{dr}$$

$$\textcircled{2} \rightarrow \frac{d}{dr} \left[ \frac{1-2\nu}{E} \sigma_{rr} + \frac{1}{2} \frac{1+\nu}{E} r \frac{d\sigma_{rr}}{dr} \right] = \frac{1}{r} \left[ \frac{-\nu}{E} r \frac{d\sigma_{rr}}{dr} - \frac{1}{2} \frac{1+\nu}{E} r \frac{d\sigma_{rr}}{dr} \right]$$

$$\frac{2(1-2\nu)}{2E} \frac{d\sigma_{rr}}{dr} + \frac{1}{2E} \frac{1+\nu}{dr} \frac{d\sigma_{rr}}{dr} + \frac{1}{2E} \frac{1+\nu}{r} r \frac{d^2\sigma_{rr}}{dr^2} = -\frac{1+\nu}{2E} \frac{d\sigma_{rr}}{dr}$$

$$(1-\nu) r \frac{d^2\sigma_{rr}}{dr^2} + \left[ 2(1-2\nu) + (1-\nu) + (1+\nu) \right] \frac{d\sigma_{rr}}{dr} = 0$$

$$\begin{matrix} 2-4\nu + 1-\nu + 1+\nu \\ 4-4\nu = 4(1-\nu) \end{matrix}$$

$$\therefore \boxed{\frac{d^2\sigma_{rr}}{dr^2} + \frac{4}{r} \frac{d\sigma_{rr}}{dr} = 0}$$

Governing equidimensional ordinary differential equation for  $\sigma_{rr}(r)$

Boundary Conditions: Traction-free void of radius  $a$   
 $\therefore \sigma_{rr}(r=a) = 0$

Hydrostatic stress as  $r \rightarrow \infty \therefore \sigma_{rr}(r \rightarrow \infty) = -\sigma_0$

Solution Procedure: Equidimensional  $\rightarrow \sigma_{rr} = A r^p$   
 $\sigma'_{rr} = A p r^{p-1}$   
 $\sigma''_{rr} = A p(p-1) r^{p-2}$

$$\therefore \cancel{A} p(p-1) r^{\cancel{p-2}} + 4 \cancel{A} p r^{\cancel{p-2}} = 0$$

$$p(p-1) + 4p = 0 \quad \leftarrow \text{characteristic equation}$$

$$p^2 + 3p = 0$$

$$p = 0, -3 \quad (\text{These are the eigenvalues of the characteristic equation})$$

$$\therefore \sigma_{rr} = A + \frac{B}{r^3} \rightarrow \sigma_{\theta\theta} = \frac{r}{2} \frac{-3B}{r^4} + A + \frac{B}{r^3}$$

$$\therefore \sigma_{\theta\theta} = A - \frac{1}{2} \frac{B}{r^3}$$

$$\sigma_{rr}(r \rightarrow \infty) = +\sigma_0 = A \quad \therefore A = +\sigma_0$$

$$\sigma_{rr}(r \rightarrow a) = 0 = +\sigma_0 + \frac{B}{a^3} \rightarrow B = -a^3 \sigma_0$$

$$\therefore \boxed{\begin{aligned} \sigma_{rr} &= +\sigma_0 \left[ 1 - \left( \frac{a}{r} \right)^3 \right] \\ \sigma_{\theta\theta} &= \sigma_0 \left[ 1 + \frac{1}{2} \left( \frac{a}{r} \right)^3 \right] \end{aligned}}$$

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## 2-D Elasticity

Plane Stress  $\rightarrow \sigma_{zz} = \sigma_{xz} = \sigma_{yz} = 0, \therefore \epsilon_{xz} = \epsilon_{yz} = 0$

$$\therefore \left. \begin{aligned} \epsilon_{xx} &= \frac{1}{E} \sigma_{xx} - \frac{\nu}{E} \sigma_{yy} \\ \epsilon_{yy} &= \frac{1}{E} \sigma_{yy} - \frac{\nu}{E} \sigma_{xx} \\ \epsilon_{xy} &= \frac{1+\nu}{E} \sigma_{xy} \end{aligned} \right\} \begin{aligned} \sigma_{xx} &= \frac{E}{1-\nu^2} \epsilon_{xx} + \frac{\nu E}{1-\nu^2} \epsilon_{yy} \\ \sigma_{yy} &= \frac{\nu E}{1-\nu^2} \epsilon_{xx} + \frac{E}{1-\nu^2} \epsilon_{yy} \\ \sigma_{xy} &= \frac{E}{1+\nu} \epsilon_{xy} \end{aligned}$$

Plane Strain  $\rightarrow \epsilon_{zz} = \epsilon_{xz} = \epsilon_{yz} = 0, \therefore \sigma_{xz} = \sigma_{yz} = 0$

$$\epsilon_{zz} = \frac{1}{E} \sigma_{zz} - \frac{\nu}{E} (\sigma_{xx} + \sigma_{yy}) = 0, \therefore \sigma_{zz} = \nu (\sigma_{xx} + \sigma_{yy})$$

$$\epsilon_{xx} = \frac{1}{E} \sigma_{xx} - \frac{\nu}{E} \sigma_{yy} - \frac{\nu}{E} \nu (\sigma_{xx} + \sigma_{yy})$$

$$\therefore \left. \begin{aligned} \epsilon_{xx} &= \frac{1-\nu^2}{E} \sigma_{xx} - \frac{\nu(1+\nu)}{E} \sigma_{yy} \\ \epsilon_{yy} &= \frac{1-\nu^2}{E} \sigma_{yy} - \frac{\nu(1+\nu)}{E} \sigma_{xx} \\ \epsilon_{xy} &= \frac{1+\nu}{E} \sigma_{xy} \end{aligned} \right\}$$

$$\sigma_{xx} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \epsilon_{xx} + \frac{E\nu}{(1+\nu)(1-2\nu)} \epsilon_{yy}$$

$$\sigma_{yy} = \frac{E(1-\nu)}{(1+\nu)(1-2\nu)} \epsilon_{yy} - \frac{E\nu}{(1+\nu)(1-2\nu)} \epsilon_{xx}$$

$$\sigma_{xy} = \frac{E}{1+\nu} \epsilon_{xy}$$

If we define  $E^*, \nu^* = \begin{cases} E, \nu & \rightarrow \text{Plane stress} \\ \frac{E}{1-\nu^2}, \frac{\nu}{1-\nu} & \rightarrow \text{Plane strain} \end{cases}$

Then

$$\left. \begin{aligned} \epsilon_{xx} &= \frac{1}{E^*} \sigma_{xx} - \frac{\nu^*}{E^*} \sigma_{yy} \\ \epsilon_{yy} &= \frac{1}{E^*} \sigma_{yy} - \frac{\nu^*}{E^*} \sigma_{xx} \\ \epsilon_{xy} &= \frac{1+\nu^*}{E^*} \sigma_{xy} \end{aligned} \right\}$$

$$\begin{aligned}\sigma_{xx} &= \frac{E^*}{1-\nu^{*2}} \epsilon_{xx} + \frac{\nu^* E^*}{1-\nu^{*2}} \epsilon_{yy} \\ \sigma_{yy} &= \frac{E^*}{1-\nu^{*2}} \epsilon_{yy} + \frac{\nu^* E^*}{1-\nu^{*2}} \epsilon_{xx} \\ \sigma_{xy} &= \frac{E^*}{1+\nu^*} \epsilon_{xy}\end{aligned}$$

check plane strain:  $\frac{E^*}{1+\nu^*} = \frac{E}{(1-\nu^2)(\frac{1-\nu}{1-\nu} + \frac{\nu}{1-\nu})} = \frac{E(1-\nu)}{(1-\nu)(1+\nu)(1-\nu)}$

Equilibrium:  $\frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} = 0$

$$\frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} = 0$$

Satisfied if  $\left. \begin{aligned}\sigma_{xx} &= \frac{\partial^2 \phi}{\partial y^2} \\ \sigma_{yy} &= \frac{\partial^2 \phi}{\partial x^2} \\ \sigma_{xy} &= -\frac{\partial^2 \phi}{\partial x \partial y}\end{aligned} \right\} \phi \equiv \text{Airy Stress Function}$

Compatibility:  $\begin{aligned}\epsilon_{xx} &= \frac{\partial u}{\partial x} \\ \epsilon_{yy} &= \frac{\partial v}{\partial y} \\ \epsilon_{xy} &= \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right)\end{aligned}$

$$\therefore \frac{\partial^2 \epsilon_{xx}}{\partial y^2} + \frac{\partial^2 \epsilon_{yy}}{\partial x^2} - 2 \frac{\partial^2 \epsilon_{xy}}{\partial x \partial y} = 0$$

$$\begin{aligned}\epsilon_{xx} &= \frac{1}{E^*} \sigma_{xx} - \frac{\nu^*}{E^*} \sigma_{yy} = \frac{1}{E^*} \frac{\partial^2 \phi}{\partial y^2} - \frac{\nu^*}{E^*} \frac{\partial^2 \phi}{\partial x^2} \\ \epsilon_{yy} &= \frac{1}{E^*} \sigma_{yy} - \frac{\nu^*}{E^*} \sigma_{xx} = \frac{1}{E^*} \frac{\partial^2 \phi}{\partial x^2} - \frac{\nu^*}{E^*} \frac{\partial^2 \phi}{\partial y^2} \\ \epsilon_{xy} &= \frac{1+\nu^*}{E^*} \sigma_{xy} = -\frac{(1+\nu^*)}{E^*} \frac{\partial^2 \phi}{\partial x \partial y}\end{aligned}$$

$$\therefore \frac{1}{E^*} \frac{\partial^4 \phi}{\partial y^4} - \frac{\nu^*}{E^*} \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + \frac{1}{E^*} \frac{\partial^4 \phi}{\partial x^4} - \frac{\nu^*}{E^*} \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + \frac{2(1+\nu^*)}{E^*} \frac{\partial^4 \phi}{\partial x^2 \partial y^2} = 0$$

$$\therefore \boxed{\frac{\partial^4 \phi}{\partial x^4} + 2 \frac{\partial^4 \phi}{\partial y^2 \partial x^2} + \frac{\partial^4 \phi}{\partial y^4} = 0}$$

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$$\nabla^4 \phi = 0 \quad \leftarrow \text{Biharmonic Equation}$$

$$\nabla^2(\nabla^2 \phi) = 0 \quad \nabla^2 = \frac{\partial^2}{\partial x^2} + \frac{\partial^2}{\partial y^2} \quad (\text{Cartesian})$$

$$\nabla^2 = \frac{\partial^2}{\partial r^2} + \frac{1}{r} \frac{\partial}{\partial r} + \frac{1}{r^2} \frac{\partial^2}{\partial \theta^2} \quad (\text{Polar})$$

$$\begin{aligned} \nabla^4 \phi = & \frac{\partial^4 \phi}{\partial r^4} + \frac{2}{r} \frac{\partial^3 \phi}{\partial r^3} - \frac{1}{r^2} \frac{\partial^3 \phi}{\partial r^2} + \frac{1}{r^3} \frac{\partial \phi}{\partial r} + \frac{2}{r^2} \frac{\partial^4 \phi}{\partial r^2 \partial \theta^2} \\ & - \frac{2}{r^3} \frac{\partial^3 \phi}{\partial r \partial \theta^2} + \frac{4}{r^4} \frac{\partial^2 \phi}{\partial \theta^2} + \frac{1}{r^4} \frac{\partial^4 \phi}{\partial \theta^4} = 0 \end{aligned}$$

$$\sigma_{rr} = \frac{1}{r} \frac{\partial \phi}{\partial r} + \frac{1}{r^2} \frac{\partial^2 \phi}{\partial \theta^2}$$

$$\sigma_{\theta\theta} = \frac{\partial^2 \phi}{\partial r^2}$$

$$\sigma_{r\theta} = -\frac{1}{r} \frac{\partial^2 \phi}{\partial r \partial \theta} + \frac{1}{r^2} \frac{\partial \phi}{\partial \theta} = -\frac{\partial}{\partial r} \left( \frac{1}{r} \frac{\partial \phi}{\partial \theta} \right)$$

We will return to polar coordinates later.

Now let's look at some solutions in Cartesian coordinates.

$$\nabla^4 \phi = \frac{\partial^4 \phi}{\partial x^4} + 2 \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + \frac{\partial^4 \phi}{\partial y^4} = 0$$

Solution by polynomials

$$\phi = \frac{A}{2} x^2 + Bxy + \frac{C}{2} y^2$$

$$\rightarrow \sigma_{xx} = C, \quad \sigma_{yy} = A, \quad \sigma_{xy} = B$$

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Cantilever Beams

Take a hint from beam theory:  $\sigma_{xx} = Ay$ ,  $\sigma_{yy} = \sigma_{xy} = 0$

$$\sigma_{xx} = \frac{\partial^2 \phi}{\partial y^2} = Ay \rightarrow \phi = \frac{1}{6} Ay^3$$

$$\begin{aligned} M &= b \int_{-h/2}^{h/2} \sigma_{xx} y dy = b \int_{-h/2}^{h/2} Ay^2 dy \\ &= b A \left[ \frac{1}{3} y^3 \right]_{-h/2}^{h/2} \\ &= A \frac{bh^3}{12} \end{aligned}$$

$$\therefore A = \frac{12}{bh^3} M$$

$$\begin{aligned} \sigma_{xx} &= \frac{12}{bh^3} M y & \left( \sigma_{xx} = \frac{My}{I} \right) \\ \sigma_{yy} &= \sigma_{xy} = 0 \end{aligned}$$

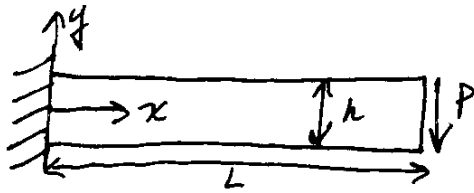
$$\begin{aligned} \epsilon_{xx} &= \frac{\partial u}{\partial x} = \frac{1}{E} \frac{12}{bh^3} M y \rightarrow u = \frac{M}{EI} yx + f(y) \\ \epsilon_{yy} &= \frac{\partial v}{\partial y} = -\frac{\nu}{E} \frac{12}{bh^3} M y \rightarrow v = -\frac{\nu M}{2EI} y^2 + g(x) \\ \epsilon_{xy} &= \frac{1}{2} \left( \frac{\partial v}{\partial y} + \frac{\partial u}{\partial x} \right) = 0 = \frac{1}{2} \left[ \frac{M}{EI} x + f'(y) + g'(x) \right] \end{aligned}$$

$$u(x=0, y) = 0 \rightarrow f(y) = 0, f'(y) = 0$$

$$\therefore g'(x) = -\frac{\nu M}{EI} x \rightarrow g(x) = -\frac{1}{2} \frac{\nu M}{EI} x^2 + C$$

$$\therefore v = -\frac{1}{2} \frac{\nu M}{EI} (y^2 + x^2) + C \xrightarrow{0} \text{b/c } v(0,0) = 0$$

$$v(x, y=0) = -\frac{1}{2} \frac{\nu M}{EI} x^2 \rightarrow \text{Agrees exactly w/ beam theory}$$



Beam theory  $\rightarrow \sigma_{xx} = \frac{M_y}{I}$ ,  $M = P(L-x)$   
 $\sigma_{xx} = A_y + Bxy = \frac{\partial^2 \phi}{\partial y^2}$

$$\therefore \phi = \frac{1}{6} A_y^3 + \frac{1}{6} Bxy^3 + Cxy$$

$$\sigma_{yy} = \frac{\partial^2 \phi}{\partial x^2} = 0, \quad \sigma_{xy} = -\frac{\partial^2 \phi}{\partial x \partial y} = -\frac{1}{2} Bxy^2 - C$$

BCs:  $\sigma_{xy} = 0$  at  $y = \pm \frac{h}{2} \rightarrow -\frac{1}{2} B \frac{h^2}{4} - C = 0$   
 $\therefore C = -\frac{Bh^2}{8}$

$$+b \int_{-\frac{h}{2}}^{\frac{h}{2}} \sigma_{xy}(x=L) dy = -P$$

$$b \int_{-\frac{h}{2}}^{\frac{h}{2}} \frac{1}{2} B \left( \frac{h^2}{4} - y^2 \right) dy = -P$$

$$\frac{b}{2} B \left( \frac{h^3}{4} - \frac{h^3}{12} \right) = -P$$

$$B \frac{bh^3}{12} = -P$$

$$B = -\frac{12P}{bh^3} = -\frac{P}{I}$$

$$\sigma_{xx}(x=L, y) = 0 = A_y + \frac{-P}{I} Ly \rightarrow A = \frac{PL}{I}$$

$$\therefore \sigma_{xx} = \frac{PL}{I} y - \frac{P}{I} xy = \frac{P(L-x)}{I} y$$

$$\sigma_{yy} = 0$$

$$\sigma_{xy} = \frac{1}{2} \frac{P}{I} \left( y^2 - \frac{h^2}{4} \right)$$



$$\epsilon_{xx} = \frac{\partial u}{\partial x} = \frac{1}{E} \frac{P(L-x)}{I} y \rightarrow u = \frac{PL}{EI} yx - \frac{1}{2} \frac{P}{EI} yx^2 + f(y)$$

$$\epsilon_{yy} = \frac{\partial v}{\partial y} = -\frac{\nu}{E} \left( \frac{PL}{I} - \frac{Px}{I} \right) y \rightarrow v = -\frac{1}{2} \nu \frac{PL}{EI} y^2 - \frac{1}{2} \nu \frac{P}{EI} xy^2 + g(x)$$

$$\epsilon_{xy} = \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) = \frac{1}{2} \left[ \frac{PL}{EI} x - \frac{1}{2} \frac{P}{EI} x^2 + f'(y) - \frac{1}{2} \nu \frac{P}{EI} y^2 + g'(x) \right]$$

$$= \frac{1+\nu}{E} \frac{1}{2} \frac{P}{I} \left( y^2 - \frac{x^2}{4} \right)$$

$$\therefore \left. \begin{aligned} g'(x) &= \frac{1}{2} \frac{P}{EI} x^2 - \frac{PL}{EI} x + C_1 \\ f'(y) &= (1+\nu) \frac{P}{EI} y^2 + \frac{1}{2} \nu \frac{P}{EI} y^2 + C_2 \end{aligned} \right\} C_1 + C_2 = -\frac{1+\nu}{E} \frac{P}{I} \frac{L^2}{4}$$

$$\therefore g(x) = \frac{1}{6} \frac{P}{EI} x^3 - \frac{1}{2} \frac{PL}{EI} x^2 + C_1 x + C_3 \quad \text{or } v(0,0)=0$$

$$v(x, y=0) = \frac{1}{6} \frac{P}{EI} x^3 - \frac{1}{2} \frac{PL}{EI} x^2 + C_1 x$$

$$\frac{\partial v}{\partial x} (x=0, y=0) = 0 \rightarrow C_1 = 0 \rightarrow C_2 = -\frac{1+\nu}{E} \frac{P}{I} \frac{L^2}{4}$$

$$\therefore v(x, y=0) = \frac{1}{6} \frac{P}{EI} x^3 - \frac{1}{2} \frac{PL}{EI} x^2 \quad (\text{Exactly the same as beam theory})$$

Note we could not enforce  $u=v=0$  over the entire built in section of the beam. Furthermore we could not enforce an arbitrary distribution of the applied loads.

### St. Venant's Principle

The stresses and strains produced in a body by the application, to a small part of its surface, of a system of forces statically equivalent to zero force and zero couple, are of negligible magnitude at distances which are large compared with the linear dimensions of the part.

### Fourier Series Solutions

$$\phi = X(x) \sin ny$$

$$D^4 \phi = X'''' \sin ny - 2n^2 X'' \sin ny + n^4 X \sin ny = 0$$

$$X = A e^{\alpha x}, \quad X'' = A \alpha^2 e^{\alpha x}, \quad X'''' = A \alpha^4 e^{\alpha x}$$

$$\therefore \alpha^4 - 2n^2 \alpha^2 + n^4 = 0$$

$$(\alpha^2 - n^2)(\alpha^2 - n^2) = 0$$

Two double roots  $\alpha = \pm n$

$$\therefore X = A_1 e^{nx} + A_2 e^{-nx} + A_3 x e^{-nx} + A_4 x e^{nx}$$

$$Y = B_1 \sin ny + B_2 \cos ny$$

# Supplement to Lecture on 9/11/01

A

## Plane Stress is an Approximation?

$$\sigma_{zz} = \sigma_{xz} = \sigma_{yz} = 0$$

$$\left. \begin{aligned} \frac{\partial \sigma_{xx}}{\partial x} + \frac{\partial \sigma_{xy}}{\partial y} &= 0 \\ \frac{\partial \sigma_{xy}}{\partial x} + \frac{\partial \sigma_{yy}}{\partial y} &= 0 \end{aligned} \right\} \begin{aligned} \sigma_{xx} &= \frac{\partial^2 \phi}{\partial y^2} \\ \sigma_{yy} &= \frac{\partial^2 \phi}{\partial x^2} \\ \sigma_{xy} &= -\frac{\partial^2 \phi}{\partial x \partial y} \end{aligned}$$

$$\left. \begin{aligned} \epsilon_{xx} &= \frac{\partial u}{\partial x} \\ \epsilon_{yy} &= \frac{\partial v}{\partial y} \\ \epsilon_{zz} &= \frac{\partial w}{\partial z} \\ \epsilon_{xy} &= \frac{1}{2} \left( \frac{\partial u}{\partial y} + \frac{\partial v}{\partial x} \right) \\ \epsilon_{xz} &= \frac{1}{2} \left( \frac{\partial u}{\partial z} + \frac{\partial w}{\partial x} \right) \\ \epsilon_{yz} &= \frac{1}{2} \left( \frac{\partial v}{\partial z} + \frac{\partial w}{\partial y} \right) \end{aligned} \right\} \begin{aligned} \epsilon_{xx, yy} + \epsilon_{yy, xx} - 2\epsilon_{xy, xy} &= 0 \quad (1) \\ \epsilon_{xx, zz} + \epsilon_{zz, xx} - 2\epsilon_{xz, xz} &= 0 \quad (2) \\ \epsilon_{yy, zz} + \epsilon_{zz, yy} - 2\epsilon_{yz, yz} &= 0 \quad (3) \\ -\epsilon_{xx, yz} - \epsilon_{yz, xx} + \epsilon_{xz, xy} + \epsilon_{xy, xz} &= 0 \quad (4) \\ -\epsilon_{yy, xz} - \epsilon_{xz, yy} + \epsilon_{yz, xy} + \epsilon_{xy, yz} &= 0 \quad (5) \\ -\epsilon_{zz, xy} - \epsilon_{xy, zz} + \epsilon_{xz, yz} + \epsilon_{yz, xz} &= 0 \quad (6) \end{aligned}$$

$$\begin{aligned} \epsilon_{xx} &= \frac{1}{E} \sigma_{xx} - \frac{\nu}{E} \sigma_{yy} = \frac{1}{E} \frac{\partial^2 \phi}{\partial y^2} - \frac{\nu}{E} \frac{\partial^2 \phi}{\partial x^2} \\ \epsilon_{yy} &= \frac{1}{E} \sigma_{yy} - \frac{\nu}{E} \sigma_{xx} = \frac{1}{E} \frac{\partial^2 \phi}{\partial x^2} - \frac{\nu}{E} \frac{\partial^2 \phi}{\partial y^2} \\ \epsilon_{zz} &= -\frac{\nu}{E} (\sigma_{xx} + \sigma_{yy}) = -\frac{\nu}{E} \left( \frac{\partial^2 \phi}{\partial x^2} + \frac{\partial^2 \phi}{\partial y^2} \right) \\ \epsilon_{xy} &= \frac{1+\nu}{E} \sigma_{xy} = -\frac{1+\nu}{E} \frac{\partial^2 \phi}{\partial x \partial y} \\ \epsilon_{xz} &= 0 \\ \epsilon_{yz} &= 0 \end{aligned}$$

$$(1) \rightarrow \frac{\partial^4 \phi}{\partial x^4} + \frac{\partial^4 \phi}{\partial y^4} + 2 \frac{\partial^4 \phi}{\partial x^2 \partial y^2} = 0 \rightarrow \nabla_{xy}^4 \phi = 0$$

$$(2) \rightarrow -\frac{\nu}{E} \left( \frac{\partial^4 \phi}{\partial x^4} + \frac{\partial^4 \phi}{\partial y^2 \partial x^2} \right) + \frac{1}{E} \frac{\partial^4 \phi}{\partial y^2 \partial x^2} - \frac{\nu}{E} \frac{\partial^4 \phi}{\partial x^2 \partial y^2} = 0$$

$$(3) \rightarrow -\frac{\nu}{E} \left( \frac{\partial^4 \phi}{\partial x^2 \partial y^2} + \frac{\partial^4 \phi}{\partial y^4} \right) + \frac{1}{E} \frac{\partial^4 \phi}{\partial x^2 \partial y^2} - \frac{\nu}{E} \frac{\partial^4 \phi}{\partial y^2 \partial x^2} = 0$$

$$(2)+(3) \rightarrow -\frac{\nu}{E} [\nabla_{xy}^4 \phi] + \frac{1-\nu}{E} \frac{\partial^2}{\partial z^2} [\nabla_{xy}^2 \phi] = 0$$

(B)

$\therefore$  from ① & ②+③ we have  $\frac{\partial^2}{\partial z^2} [\nabla_{xy}^2 \phi] = 0$

$$\textcircled{4} \rightarrow \frac{\partial}{\partial z} \left[ \frac{\partial}{\partial y} (\nabla_{xy}^2 \phi) \right] = 0$$

$$\textcircled{5} \rightarrow \frac{\partial}{\partial z} \left[ \frac{\partial}{\partial x} (\nabla_{xy}^2 \phi) \right] = 0$$

$$\therefore \nabla_{xy}^2 \phi = Cz + \theta_0(x, y)$$

$$\text{and } \textcircled{1} \rightarrow \nabla_{xy}^2 \theta_0 = 0$$

$$\textcircled{6} \rightarrow -\nu \frac{\partial^2}{\partial x \partial y} (\nabla_{xy}^2 \phi) = (1+\nu) \frac{\partial^2}{\partial z^2} \left( \frac{\partial^2 \phi}{\partial x \partial y} \right) = (1+\nu) \frac{\partial}{\partial x \partial y} \left( \frac{\partial^2 \phi}{\partial z^2} \right)$$

$$-\nu \frac{\partial^2}{\partial x \partial y} [Cz + \theta_0(x, y)] = (1+\nu) \frac{\partial^2}{\partial x \partial y} \frac{\partial^2 \phi}{\partial z^2}$$

$$-\nu [Cz + \theta_0(x, y)] = (1+\nu) \frac{\partial^2 \phi}{\partial z^2} + a(z)x + b(z)y + d(z)$$

$$\therefore \phi = -\frac{\nu}{1+\nu} \left[ \frac{1}{3} Cz^3 + \frac{1}{2} z^2 \theta_0(x, y) \right] + A(z)x + B(z)y + D(z) \\ + \phi_1(x, y)z + \phi_0(x, y)$$

Note that stresses are unaffected if  $C = A(z) = B(z) = D(z) = 0$

Furthermore, if stresses are symmetric about the midplane, i.e.  $\sigma_{ij}(z) = \sigma_{ij}(-z)$ , then  $\phi_1(x, y) = 0$

$$\therefore \phi = \phi_0(x, y) - \frac{1}{2} \frac{\nu}{1+\nu} z^2 \theta_0(x, y)$$

(C)

$$\text{but } \nabla_{xy}^2 \phi = \theta_0(x, y)$$

$$\nabla_{xy}^2 \phi = \nabla_{xy}^2 \phi_0 - \frac{1}{2} \frac{\nu}{1+\nu} z^2 \nabla_{xy}^2 \theta_0$$

$$\text{but } \nabla_{xy}^2 \theta_0 = 0 \rightarrow \nabla_{xy}^2 \phi = \nabla_{xy}^2 \phi_0$$

$$\text{and } \theta_0(x, y) = \nabla_{xy}^2 \phi_0$$

~~$$\therefore \phi = \phi_0(x, y) - \frac{1}{2} \frac{\nu}{1+\nu} z^2 \nabla_{xy}^2 \phi_0$$~~

$$\therefore \boxed{\phi = \phi_0(x, y) - \frac{1}{2} \frac{\nu}{1+\nu} z^2 \nabla_{xy}^2 \phi_0}$$

$$\text{and } \nabla_{xy}^4 \phi = 0$$

$$\rightarrow \nabla_{xy}^4 \phi_0 - \frac{1}{2} \frac{\nu}{1+\nu} z^2 \nabla_{xy}^2 (\nabla_{xy}^2 \theta_0) = 0$$

$$\therefore \boxed{\nabla_{xy}^4 \phi_0 = 0}$$

Hence, stresses have a parabolic distribution in the  $z$  direction. But, if the thickness of the plate is very small then the contribution to the stresses is very small due to the  $z$ -varying term.

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# Back to Polar Coordinates

Solutions of the form  $\phi = R(r) \cos n\theta$

$$R'''' \cos n\theta + \frac{2}{r} R''' \cos n\theta - \frac{1}{r^2} R'' \cos n\theta + \frac{1}{r^3} R' \cos n\theta - \frac{1}{r^2} 2n^2 R'' \cos n\theta + \frac{1}{r^3} 2n^2 R' \cos n\theta - \frac{1}{r^4} 4n^2 R \cos n\theta + \frac{1}{r^4} n^4 R \cos n\theta = 0$$

$$R'''' + \frac{2}{r} R''' - \frac{1}{r^2} (2n^2 + 1) R'' + \frac{1}{r^3} (2n^2 + 1) R' + \frac{1}{r^4} n^2 (n^2 - 4) R = 0$$

Equidimensional  $\rightarrow R = A r^p, R' = A p r^{p-1}, R'' = A p(p-1) r^{p-2}$   
 $R''' = A p(p-1)(p-2) r^{p-3}, R'''' = A p(p-1)(p-2)(p-3) r^{p-4}$

$$\therefore p(p-1)(p-2)(p-3) + 2p(p-1)(p-2) - (2n^2 + 1)p(p-1) + (2n^2 + 1)p + n^2(n^2 - 4) = 0$$

$$(p-n)(p+n)[p-(n+2)][p+(n-2)] = 0$$

$$\therefore p = \pm n, p = n+2, p = 2-n$$

Double roots when  $n=0 \rightarrow R = A \ln r$   
 and  $R = A r^2 \ln r$   
 are solutions

$$n = \pm 1 \rightarrow R = A r \ln r$$

is a solution

## POLAR COMPONENTS OF STRESS AND DISPLACEMENT

$$\kappa = 3 - 4\nu$$

$$\kappa = (3 - \nu)/(1 + \nu)$$

for plane strain,  
for plane stress

Rigid body displacements:

$$u_r = C_1 \cos \theta + C_2 \sin \theta$$

$$u_\theta = -C_1 \sin \theta + C_2 \cos \theta + C_3 r$$

$u = \phi$	$\sigma_{rr}$	$\sigma_{r\theta}$	$\sigma_{\theta\theta}$	$2Gu_r$	$2Gu_\theta$
$r^2$	2	0	2	$(\kappa-1)r$	0
$\log r$	$1/r^2$	0	$-1/r^2$	$-1/r$	0
$\theta$	0	$1/r^2$	0	0	$-1/r$
$r^2 \log r$	$2 \log r + 1$	0	$2 \log r + 3$	$(\kappa-1)r \log r - r$	$(\kappa+1)r\theta$
$r^2 \theta$	$2\theta$	-1	$2\theta$	$(\kappa-1)r\theta$	$-(\kappa+1)r \log r$
$r^3 \cos \theta$	$2r \cos \theta$	$2r \sin \theta$	$6r \cos \theta$	$(\kappa-2)r^2 \cos \theta$	$(\kappa+2)r^2 \sin \theta$
$r^3 \sin \theta$	$2r \sin \theta$	$-2r \cos \theta$	$6r \sin \theta$	$(\kappa-2)r^2 \sin \theta$	$-(\kappa+2)r^2 \cos \theta$
$r \theta \sin \theta$	$2 \cos \theta / r$	0	0	$\frac{1}{2}[(\kappa-1)\theta \sin \theta + (\kappa+1) \log r \cos \theta - \cos \theta]$	$\frac{1}{2}[(\kappa-1)\theta \cos \theta - (\kappa+1) \log r \sin \theta - \sin \theta]$
$r \theta \cos \theta$	$-2 \sin \theta / r$	0	0	$\frac{1}{2}[(\kappa-1)\theta \cos \theta - (\kappa+1) \log r \sin \theta + \sin \theta]$	$\frac{1}{2}[-(\kappa-1)\theta \sin \theta - (\kappa+1) \log r \cos \theta - \cos \theta]$
$r \log r \cos \theta$	$\cos \theta / r$	$\sin \theta / r$	$\cos \theta / r$	$\frac{1}{2}[(\kappa+1)\theta \sin \theta + (\kappa-1) \log r \cos \theta - \cos \theta]$	$\frac{1}{2}[(\kappa+1)\theta \cos \theta - (\kappa-1) \log r \sin \theta - \sin \theta]$
$r \log r \sin \theta$	$\sin \theta / r$	$-\cos \theta / r$	$\sin \theta / r$	$\frac{1}{2}[-(\kappa+1)\theta \cos \theta + (\kappa-1) \log r \sin \theta - \sin \theta]$	$\frac{1}{2}[(\kappa+1)\theta \sin \theta + (\kappa-1) \log r \cos \theta + \cos \theta]$
$\cos \theta / r$	$-2 \cos \theta / r^3$	$-2 \sin \theta / r^3$	$2 \cos \theta / r^3$	$\cos \theta / r^2$	$\sin \theta / r^2$
$\sin \theta / r$	$-2 \sin \theta / r^3$	$2 \cos \theta / r^3$	$2 \sin \theta / r^3$	$\sin \theta / r^2$	$-\cos \theta / r^2$

$U = \Phi$	$\sigma_{rr}$	$\sigma_{r\theta}$	$\sigma_{\theta\theta}$	$2G u_r$	$2G u_\theta$
$r^2 \cos 2\theta$	$-2 \cos 2\theta$	$2 \sin 2\theta$	$2 \cos 2\theta$	$-2r \cos 2\theta$	$2r \sin 2\theta$
$r^2 \sin 2\theta$	$-2 \sin 2\theta$	$-2 \cos 2\theta$	$2 \sin 2\theta$	$-2r \sin 2\theta$	$-2r \cos 2\theta$
$r^4 \cos 2\theta$	0	$6r^2 \sin 2\theta$	$12r^2 \cos 2\theta$	$-(3-\kappa)r^3 \cos 2\theta$	$(3+\kappa)r^3 \sin 2\theta$
$r^4 \sin 2\theta$	0	$-6r^2 \cos 2\theta$	$12r^2 \sin 2\theta$	$-(3-\kappa)r^3 \sin 2\theta$	$-(3+\kappa)r^3 \cos 2\theta$
$\cos 2\theta/r^2$	$-6 \cos 2\theta/r^4$	$-6 \sin 2\theta/r^4$	$6 \cos 2\theta/r^4$	$2 \cos 2\theta/r^3$	$2 \sin 2\theta/r^3$
$\sin 2\theta/r^2$	$-6 \sin 2\theta/r^4$	$6 \cos 2\theta/r^4$	$6 \sin 2\theta/r^4$	$2 \sin 2\theta/r^3$	$-2 \cos 2\theta/r^3$
$\cos 2\theta$	$-4 \cos 2\theta/r^2$	$-2 \sin 2\theta/r^2$	0	$(\kappa+1) \cos 2\theta/r$	$-(\kappa-1) \sin 2\theta/r$
$\sin 2\theta$	$-4 \sin 2\theta/r^2$	$2 \cos 2\theta/r^2$	0	$(\kappa+1) \sin 2\theta/r$	$(\kappa-1) \cos 2\theta/r$
$r^n \cos n\theta$	$-n(n-1)r^{n-2} \cos n\theta$	$n(n-1)r^{n-2} \sin n\theta$	$n(n-1)r^{n-2} \cos n\theta$	$-nr^{n-1} \cos n\theta$	$nr^{n-1} \sin n\theta$
$r^n \sin n\theta$	$-n(n-1)r^{n-2} \sin n\theta$	$-n(n-1)r^{n-2} \cos n\theta$	$n(n-1)r^{n-2} \sin n\theta$	$-nr^{n-1} \sin n\theta$	$-nr^{n-1} \cos n\theta$
$r^{n+2} \cos n\theta$	$-(n+1)(n-2)r^n \cos n\theta$	$(n+1)nr^n \sin n\theta$	$(n+2)(n+1)r^n \cos n\theta$	$-(n+1-\kappa)r^{n+1} \cos n\theta$	$(n+1+\kappa)r^{n+1} \sin n\theta$
$r^{n+2} \sin n\theta$	$-(n+1)(n-2)r^n \sin n\theta$	$-(n+1)nr^n \cos n\theta$	$(n+2)(n+1)r^n \sin n\theta$	$-(n+1-\kappa)r^{n+1} \sin n\theta$	$-(n+1+\kappa)r^{n+1} \cos n\theta$
$\cos n\theta/r^n$	$-(n+1)n \cos n\theta/r^{n+2}$	$-(n+1)n \sin n\theta/r^{n+2}$	$(n+1)n \cos n\theta/r^{n+2}$	$n \cos n\theta/r^{n+1}$	$n \sin n\theta/r^{n+1}$
$\sin n\theta/r^n$	$-(n+1)n \sin n\theta/r^{n+2}$	$(n+1)n \cos n\theta/r^{n+2}$	$(n+1)n \sin n\theta/r^{n+2}$	$n \sin n\theta/r^{n+1}$	$-n \cos n\theta/r^{n+1}$
$\cos n\theta/r^{n-2}$	$-(n+2)(n-1) \cos n\theta/r^n$	$-n(n-1) \sin n\theta/r^n$	$(n-1)(n-2) \cos n\theta/r^n$	$(n-1+\kappa) \cos n\theta/r^{n-1}$	$(n-1-\kappa) \sin n\theta/r^{n-1}$
$\sin n\theta/r^{n-2}$	$-(n+2)(n-1) \sin n\theta/r^n$	$n(n-1) \cos n\theta/r^n$	$(n-1)(n-2) \sin n\theta/r^n$	$(n-1+\kappa) \sin n\theta/r^{n-1}$	$-(n-1-\kappa) \cos n\theta/r^{n-1}$



## Radially Symmetric Solutions

No  $\theta$  dependence  $\rightarrow n=0$  for  $\phi = R(r) \cos n\theta$   
in  $\phi$

governing equation:  $R'''' + \frac{2}{r}R''' - \frac{1}{r^2}R'' + \frac{1}{r^3}R' = 0$   
 $R = Ar^p$

$p=0, p=2$  (double roots)

$\therefore R(r) = \phi = \cancel{A} + B \ln r + C r^2 + D r^2 \ln r$   
 $\rightarrow$  does not affect stresses or strains

$\sigma_{rr} = B/r^2 + 2C + 2D \ln r + D$

$\sigma_{\theta\theta} = -B/r^2 + 2C + 2D \ln r + 3D$

$\sigma_{r\theta} = 0$  (as we would expect for radial symmetry)

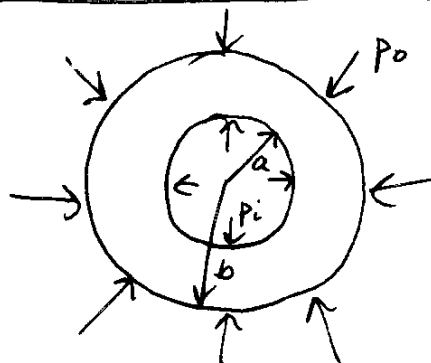
$2\chi u_r = -B/r + C(\chi-1)r + D(\chi-1)r \ln r - Dr$

$2\chi u_\theta = D(\chi+1)r\theta \leftarrow \text{Note } D \text{ leads to } \theta \text{ dependence in } u_\theta$

$\chi = \begin{cases} \frac{3-4\nu}{1+\nu} & \text{plane strain} \\ \frac{3-\nu}{1+\nu} & \text{plane stress} \end{cases}$

$D \neq 0$  for pure bending of a curved bar

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Pressure Vessel (no  $\theta$  dependence in  $u_\theta$ )

$$\sigma_{rr}(r=a) = -p_i$$

$$\sigma_{rr}(r=b) = -p_o$$

$$\sigma_{rr}(r=a) = B/a^2 + 2C = -p_i$$

$$\sigma_{rr}(r=b) = B/b^2 + 2C = -p_o$$

$$\frac{B(b^2 - a^2)}{a^2 b^2} = p_o - p_i$$

$$B = \frac{(p_o - p_i) a^2 b^2}{b^2 - a^2}$$

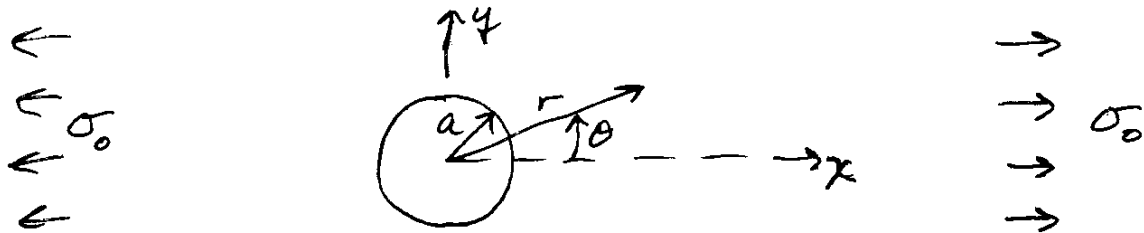
$$\therefore \frac{(p_o - p_i) b^2}{b^2 - a^2} + 2C = -p_i = \frac{-p_i b^2 + p_i a^2}{b^2 - a^2}$$

$$\frac{1}{2} 2C = \frac{1}{2} \frac{p_i a^2 - p_o b^2}{b^2 - a^2}$$

$$\therefore \sigma_{rr} = \frac{p_o - p_i}{1 - (a/b)^2} \left(\frac{a}{r}\right)^2 + \frac{p_i (a/b)^2 - p_o}{1 - (a/b)^2}$$

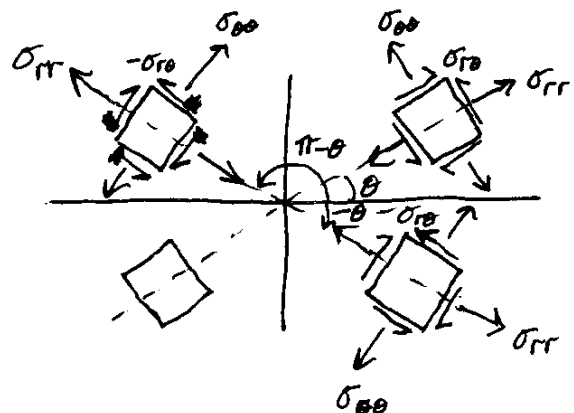
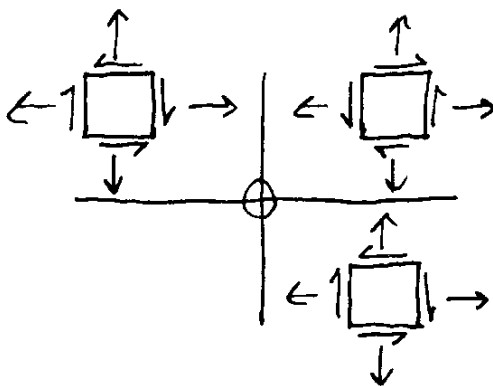
$$\sigma_{\theta\theta} = \frac{p_i - p_o}{1 - (a/b)^2} \left(\frac{a}{r}\right)^2 + \frac{p_i (a/b)^2 - p_o}{1 - (a/b)^2}$$

# Hole in an Infinite Plate Subject to Simple Tension



BCs:  $\sigma_{rr} = \sigma_{r\theta} = 0$  at  $r = a$ , all  $\theta$   
 $\sigma_{xx} = \sigma_0$ ,  $\sigma_{yy} = \sigma_{xy} = 0$  as  $r \rightarrow \infty$

Symmetry:  $\sigma_{xx}(x, y) = \sigma_{xx}(x, -y)$   
 $\sigma_{yy}(x, y) = \sigma_{yy}(x, -y)$   
 $\sigma_{xy}(x, y) = -\sigma_{xy}(x, -y)$   
 $\sigma_{rr}(x, y) = \sigma_{rr}(x, -y)$ ,  $\sigma_{rr}(r, \theta) = \sigma_{rr}(r, \pi - \theta)$   
 $\sigma_{\theta\theta}(x, y) = \sigma_{\theta\theta}(x, -y)$ ,  $\sigma_{\theta\theta}(r, \theta) = \sigma_{\theta\theta}(r, \pi - \theta)$   
 $\sigma_{r\theta}(x, y) = -\sigma_{r\theta}(x, -y)$ ,  $\sigma_{r\theta}(r, \theta) = -\sigma_{r\theta}(r, \pi - \theta)$



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Note that for  $\phi = R_{cn}(r) \cos n\theta + R_{sn}(r) \sin n\theta$

$$\begin{aligned}\sigma_{rr} &= \sigma_{rr}^{cn}(r) \cos n\theta + \sigma_{rr}^{sn}(r) \sin n\theta \\ \sigma_{\theta\theta} &= \sigma_{\theta\theta}^{cn}(r) \cos n\theta + \sigma_{\theta\theta}^{sn}(r) \sin n\theta \\ \sigma_{r\theta} &= \sigma_{r\theta}^{cn}(r) \sin n\theta + \sigma_{r\theta}^{sn}(r) \cos n\theta\end{aligned}$$

$$\sigma_{rr}(r, \theta) = \sigma_{rr}(r, -\theta), \sigma_{\theta\theta}(r, \theta) = \sigma_{\theta\theta}(r, -\theta), \sigma_{r\theta}(r, \theta) = -\sigma_{r\theta}(r, -\theta)$$

$\rightarrow$  we want  $\phi = R_c(r) \cos n\theta$  terms only

$$\sigma_{\theta\theta} = \frac{\partial^2 \phi}{\partial r^2} = -n R_{cn}''(r) \cos n\theta$$

$$\sigma_{\theta\theta}(r, \theta) = \sigma_{\theta\theta}(r, \pi - \theta) \rightarrow -n R_{cn}''(r) \cos n\theta = -n R_{cn}''(r) \cos n(\pi - \theta)$$

$$\cos n\theta = \cos n(\pi - \theta) = \cos(n\pi - n\theta)$$

$$\cos n\theta = \cos n\pi \cos n\theta + \sin n\pi \sin n\theta$$

$$\therefore \text{we want } \cancel{\cos n\pi} \cos n\pi = 0, 2\pi, 4\pi, 6\pi \dots$$

$$\therefore n = 0, 2, 4, 6 \dots$$

$\therefore$  The most general solution for this type of symmetry has the form

$$\phi = \sum_{m=0}^{\infty} R_{2m}(r) \cos 2m\theta$$

1st term:  $R_0(r) = B \ln r + Cr^2 + Dr^2 \ln r$

$$u_\theta(r, \frac{\pi}{2}) = u_\theta(r, \frac{-\pi}{2}) \rightarrow D = 0$$

2nd term:  $R_2(r) = Er^2 + F/r^2 + Gr^4 + H$

$$\phi = B \ln r + C r^2 + E r^2 \cos 2\theta + F \frac{1}{r^2} \cos 2\theta + G r^4 \cos 2\theta + H \cos 2\theta$$

$$\sigma_{rr} = \frac{B}{r^2} + 2C - 2E \cos 2\theta - 6F \frac{1}{r^4} \cos 2\theta - 4H \frac{1}{r^2} \cos 2\theta$$

$$\sigma_{\theta\theta} = -\frac{B}{r^2} + 2C + 2E \cos 2\theta + 6F \frac{1}{r^4} \cos 2\theta + 12G r^2 \cos 2\theta$$

$$\sigma_{r\theta} = 2E \sin 2\theta - 6F \frac{1}{r^4} \sin 2\theta + 6G r^2 \sin 2\theta - 2H \frac{1}{r^2} \sin 2\theta$$

$$\begin{aligned} \text{at } \infty : \sigma_{rr} &= \sigma_{xx} \cos^2 \theta = \frac{1}{2} \sigma_{xx} (1 + \cos 2\theta) = \frac{\sigma_0}{2} (1 + \cos 2\theta) \\ \sigma_{\theta\theta} &= \sigma_{xx} \sin^2 \theta = \frac{\sigma_0}{2} (1 - \cos 2\theta) \\ \sigma_{r\theta} &= -\sigma_{xx} \sin \theta \cos \theta = -\frac{\sigma_0}{2} \sin 2\theta \end{aligned}$$

all stresses finite as  $r \rightarrow \infty \rightarrow G = 0$

$$\begin{aligned} \therefore \text{for } r \rightarrow \infty \quad 2C - 2E \cos 2\theta &= \frac{\sigma_0}{2} + \frac{\sigma_0}{2} \cos 2\theta \\ 2C + 2E \cos 2\theta &= \frac{\sigma_0}{2} - \frac{\sigma_0}{2} \cos 2\theta \\ 2E \sin 2\theta &= -\frac{\sigma_0}{2} \sin 2\theta \end{aligned}$$

$$\therefore C = \frac{\sigma_0}{4}, \quad E = -\frac{\sigma_0}{4}$$

$$\text{at } r = a : \sigma_{rr} = \frac{B}{a^2} + \frac{\sigma_0}{2} + \left( \frac{\sigma_0}{2} - \frac{6F}{a^4} - \frac{4H}{a^2} \right) \cos 2\theta = 0$$

$$\sigma_{\theta\theta} = \left( -\frac{\sigma_0}{2} - \frac{6F}{a^4} - \frac{2H}{a^2} \right) \sin 2\theta = 0$$

$$\text{Must be true for all } \theta \rightarrow B = -\frac{\sigma_0 a^2}{2}$$

$$\text{and } \frac{\sigma_0}{2} - \frac{6F}{a^4} - \frac{4H}{a^2} = 0, \quad -\frac{\sigma_0}{2} - \frac{6F}{a^4} - \frac{2H}{a^2} = 0$$

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$$\therefore \sigma_0 - \frac{2H}{a^2} = 0 \rightarrow H = \frac{\sigma_0 a^2}{2}$$

$$\frac{\sigma_0}{2} - \frac{6F}{a^4} - \frac{4\sigma_0}{2} = 0 \rightarrow F = \frac{-\sigma_0 a^4}{4}$$

$$\text{Then: } \sigma_{rr} = \frac{\sigma_0}{2} \left[ 1 - \left( \frac{a}{r} \right)^2 \right] + \frac{\sigma_0}{2} \left[ 1 - 4 \left( \frac{a}{r} \right)^2 + 3 \left( \frac{a}{r} \right)^4 \right] \cos 2\theta$$

$$\sigma_{\theta\theta} = \frac{\sigma_0}{2} \left[ 1 + \left( \frac{a}{r} \right)^2 \right] + \frac{\sigma_0}{2} \left[ -1 - 3 \left( \frac{a}{r} \right)^4 \right] \cos 2\theta$$

$$\sigma_{r\theta} = \frac{\sigma_0}{2} \left[ -1 - 2 \left( \frac{a}{r} \right)^2 + 3 \left( \frac{a}{r} \right)^4 \right] \sin 2\theta$$

### Stress concentrations

$$\sigma_{\theta\theta} (r=a, \theta=\pm \frac{\pi}{2}) = \sigma_0 - 2\sigma_0 \cos \pi = 3\sigma_0$$

$$\sigma_{\theta\theta} (r=a, \theta=0, \pi) = \sigma_0 - 2\sigma_0 \cos 2\pi = -\sigma_0$$

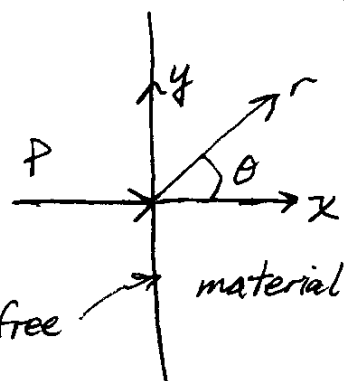
Let's say a glass material has a tensile strength of  $\sigma_c$ .

Then a glass plate with a hole in it will break under tension when  $\sigma_0 = \sigma_c/3$  and under compression when the magnitude of the applied compressive stress reaches  $\sigma_c$ .

9/18/01

Point Loads

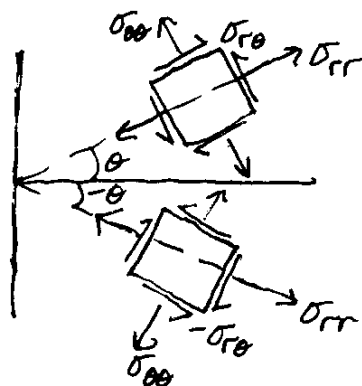
Point Load on a half space



Dimensional Analysis:  $P = \frac{F}{L}$  Traction free  
 $\sigma_{ij} = \frac{F}{L^2} \rightarrow \sigma_{ij} \sim \frac{P}{r}$

Look for  $\phi$  that give stresses like  $\frac{1}{r}$ Symmetry

$$\begin{cases} \sigma_{rr}(r, \theta) = \sigma_{rr}(r, -\theta) \\ \sigma_{\theta\theta}(r, \theta) = \sigma_{\theta\theta}(r, -\theta) \\ \sigma_{r\theta}(r, \theta) = -\sigma_{r\theta}(r, -\theta) \end{cases}$$



Look for even functions for  $\sigma_{rr}, \sigma_{\theta\theta}$ , odd for  $\sigma_{r\theta}$

$$\begin{aligned} \text{BC's: } \sigma_{\theta\theta}(r \neq 0, \theta = \pm \frac{\pi}{2}) &= 0 \\ \sigma_{r\theta}(r \neq 0, \theta = \pm \frac{\pi}{2}) &= 0 \\ \sigma_{rr} = \sigma_{r\theta} = \sigma_{\theta\theta} &= 0 \text{ as } r \rightarrow \infty \end{aligned}$$

$$\phi = A r \theta \sin \theta + B r \ln r \cos \theta$$

$$\sigma_{rr} = 2A \frac{1}{r} \cos \theta + B \frac{1}{r} \cos \theta$$

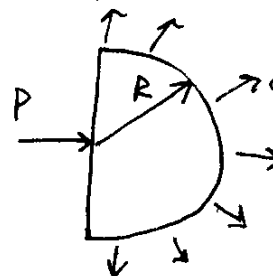
$$\sigma_{\theta\theta} = B \frac{1}{r} \cos \theta$$

$$\sigma_{r\theta} = B \frac{1}{r} \sin \theta$$

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$$\sigma_{r\theta}(r \neq 0, \theta = \pm \frac{\pi}{2}) = \pm \frac{B}{r} = 0 \rightarrow B = 0$$

$$\therefore \sigma_{rr} = 2A \frac{1}{r} \cos \theta, \quad \sigma_{\theta\theta} = \sigma_{r\theta} = 0$$



$$P + \int_{-\pi/2}^{\pi/2} T_x R d\theta = 0$$

$$T_x = \sigma_{xx} n_x + \sigma_{xy} n_y$$

$$\sigma_{xx} = \sigma_{rr} \cos^2 \theta \quad n_x = \cos \theta$$

$$\sigma_{xy} = \sigma_{rr} \sin \theta \cos \theta \quad n_y = \sin \theta$$

$$\therefore T_x = (\sigma_{rr} \cos^2 \theta + \sigma_{rr} \sin^2 \theta) \cos \theta = \sigma_{rr} \cos \theta$$

$$\therefore P + \int_{-\pi/2}^{\pi/2} 2A \frac{1}{R} \cos^2 \theta R d\theta = 0$$

$$P + 2A \int_{-\pi/2}^{\pi/2} \frac{1}{2} (1 + \cos 2\theta) d\theta = 0$$

$$P + A \left[ \theta + \frac{1}{2} \sin 2\theta \right]_{-\pi/2}^{\pi/2} = 0$$

$$P + A\pi = 0 \rightarrow A = -\frac{P}{\pi}$$

$$\therefore \sigma_{rr} = -\frac{2P}{\pi} \frac{1}{r} \cos \theta, \quad \sigma_{r\theta} = \sigma_{\theta\theta} = 0$$

$$u_r = \frac{-P}{4\pi\mu} \left[ (x-1)\theta \sin \theta + (x+1) \ln r \cos \theta - \cos \theta \right] + C_1 \cos \theta$$

$$u_\theta = \frac{-P}{4\pi\mu} \left[ (x-1)\theta \cos \theta - (x+1) \ln r \sin \theta - \sin \theta \right] - C_1 \sin \theta$$



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$$u_\theta(r, \theta = \pm\pi/2) = \frac{\pm P}{4\pi\mu} [(X+1)\ln r + 1] \pm C_1$$

Let's pick  $u_\theta = 0$  at some distance  $d$  from the origin.

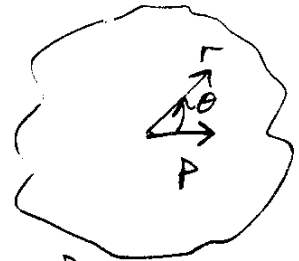
$$\text{Then } \pm \frac{P}{4\pi\mu} [(X+1)\ln d + 1] \pm C_1 = 0$$

$$C_1 = [-(X+1)\ln d - 1] \frac{P}{4\pi\mu}$$

$$\therefore u_\theta = -\frac{P}{4\pi\mu} [(X-1)\theta \cos\theta + (X+1)\ln\left(\frac{d}{r}\right)\sin\theta]$$

$$u_r = -\frac{P}{4\pi\mu} [(X-1)\theta \sin\theta - (X+1)\ln\left(\frac{d}{r}\right)\cos\theta]$$

### Point Load In Infinite Space



Can we use the same solution as the half space but with  $\sigma_{rr} = \frac{-P}{\pi} \frac{1}{r} \cos\theta$ ?

Only BCs are that  $\sigma_{ij} \rightarrow 0$  as  $r \rightarrow \infty$

$$\text{and } P + \int_{-\pi}^{\pi} T_x R d\theta = 0 \rightarrow A = \frac{-P}{2\pi}$$

However we must also have  $u_r(r, \pi) = u_r(r, -\pi)$   
 $u_\theta(r, \pi) = u_\theta(r, -\pi)$

Check

$$u_r(r, \pm\pi) = \frac{-P}{2\pi\mu} [(X+1)\ln\left(\frac{d}{r}\right)] \quad \checkmark \text{ continuous}$$

$$u_\theta(r, \pm\pi) = \frac{-P}{2\pi\mu} [(X-1)(\mp\pi)] \quad \times \text{ not continuous}$$

Return to general solution:  $\phi = A r \theta \sin \theta + B r \ln r \cos \theta$

$$\sigma_{rr} = (2A+B) \frac{1}{r} \cos \theta$$

$$\sigma_{\theta\theta} = B \frac{1}{r} \cos \theta$$

$$\sigma_{r\theta} = B \frac{1}{r} \sin \theta$$

Again  $T_x = \sigma_{xx} n_x + \sigma_{xy} n_y$ ,  $n_x = \cos \theta$ ,  $n_y = \sin \theta$

$$\sigma_{xx} = \sigma_{rr} \cos^2 \theta + \sigma_{\theta\theta} \sin^2 \theta - 2\sigma_{r\theta} \sin \theta \cos \theta$$

$$\sigma_{xy} = \sigma_{rr} \sin \theta \cos \theta - \sigma_{\theta\theta} \sin \theta \cos \theta + \sigma_{r\theta} (\cos^2 \theta - \sin^2 \theta)$$

$$T_x = \sigma_{rr} (\cos^3 \theta + \sin^2 \theta \cos \theta) + \sigma_{\theta\theta} (\sin^2 \theta \cos \theta - \sin^2 \theta \cos \theta) + \sigma_{r\theta} (\cos^3 \theta \sin \theta - \sin^3 \theta - \sin \theta \cos^3 \theta)$$

$$T_x = \sigma_{rr} \cos \theta - \sigma_{r\theta} \sin \theta$$

$$\therefore P + \int_{-\pi}^{\pi} [(2A+B) \frac{1}{r} \cos^2 \theta - B \frac{1}{r} \sin^2 \theta] r d\theta = 0$$

$$P + (2A+B)\pi - B\pi = 0 \rightarrow A = \frac{-P}{2\pi}$$

Displacement Continuity at  $\theta = \pm\pi$

$$2u_r = \frac{A}{2} [(X-1)\theta \sin \theta + (X+1) \ln r \cos \theta - \cos \theta] + \frac{B}{2} [(X+1)\theta \sin \theta + (X-1) \ln r \cos \theta - \cos \theta] + RBD$$

$$2u_\theta = \frac{A}{2} [(X-1)\theta \cos \theta - (X+1) \ln r \sin \theta - \sin \theta] + \frac{B}{2} [(X+1)\theta \cos \theta - (X-1) \ln r \sin \theta - \sin \theta] + RBD$$

$$2u_r(r, \pm\pi) = \frac{A}{2}[-(x+1)\ln r + 1] + \frac{B}{2}[-(x-1)\ln r + 1] + RBD$$

$$2u_\theta(r, \pm\pi) = \mp \frac{A}{2}(x-1)\pi \mp \frac{B}{2}(x+1)\pi + RBD$$

$$\therefore A(x-1) + B(x+1) = 0$$

$$B = -A \frac{x-1}{x+1} = \frac{P}{2\pi} \frac{x-1}{x+1}$$

$$\therefore \sigma_{rr} = -\frac{P}{2\pi} \frac{x+3}{x+1} \frac{1}{r} \cos\theta$$

$$\sigma_{\theta\theta} = \frac{P}{2\pi} \frac{x-1}{x+1} \frac{1}{r} \cos\theta$$

$$\sigma_{r\theta} = \frac{P}{2\pi} \frac{x-1}{x+1} \frac{1}{r} \sin\theta$$

### Cartesian Components

Point Load on Half-Space (2-D)

$$\sigma_{xx} = -\frac{2P}{\pi} \frac{1}{r} \cos^3\theta, \quad \sigma_{xy} = -\frac{2P}{\pi} \frac{1}{r} \sin\theta \cos^2\theta$$

$$\sigma_{yy} = -\frac{2P}{\pi} \frac{1}{r} \sin^2\theta \cos\theta$$

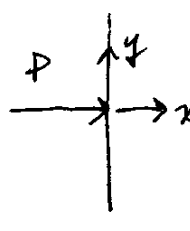
Point Load in Infinite Space (2-D)

$$\sigma_{xx} = -\frac{P}{2\pi} \frac{1}{r} \cos\theta \frac{[(x+3)\cos^2\theta + (x-1)\sin^2\theta]}{x+1}$$

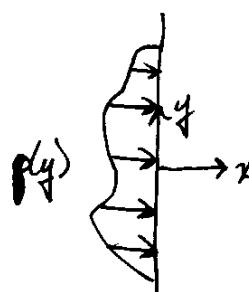
$$\sigma_{yy} = \frac{P}{2\pi} \frac{1}{r} \cos\theta \frac{[(x-1)\cos^2\theta + (x+3)\sin^2\theta]}{x+1}$$

$$\sigma_{xy} = \frac{P}{2\pi} \frac{1}{r} \sin\theta \frac{[-(x-1)\sin^2\theta - (x+3)\cos^2\theta]}{x+1}$$

Using Point Forces to Determine Stress fields for distributed loads



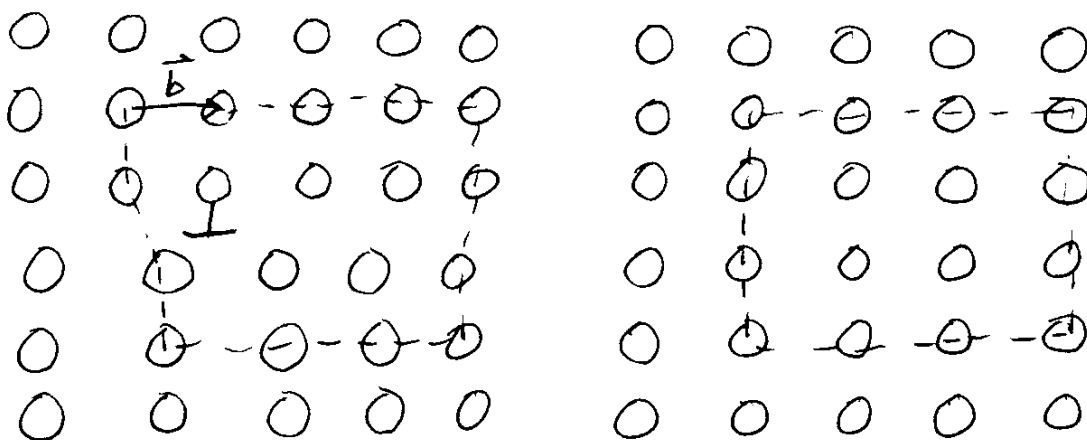
$$\left. \begin{aligned} \sigma_{xx} &= -\frac{2P}{\pi} \frac{x^3}{r^4} \\ \sigma_{yy} &= -\frac{2P}{\pi} \frac{xy^2}{r^4} \\ \sigma_{xy} &= -\frac{2P}{\pi} \frac{x^2y}{r^4} \end{aligned} \right\} r^4 = (x^2 + y^2)^2$$



$$\begin{aligned} \sigma_{xx} &= -\frac{2}{\pi} \int \frac{x^3}{[x^2 + (y - y')^2]^2} p(y') dy' \\ \sigma_{yy} &= -\frac{2}{\pi} \int \frac{x(y - y')^2}{[x^2 + (y - y')^2]^2} p(y') dy' \\ \sigma_{xy} &= -\frac{2}{\pi} \int \frac{x^2(y - y')}{[x^2 + (y - y')^2]^2} p(y') dy' \end{aligned}$$

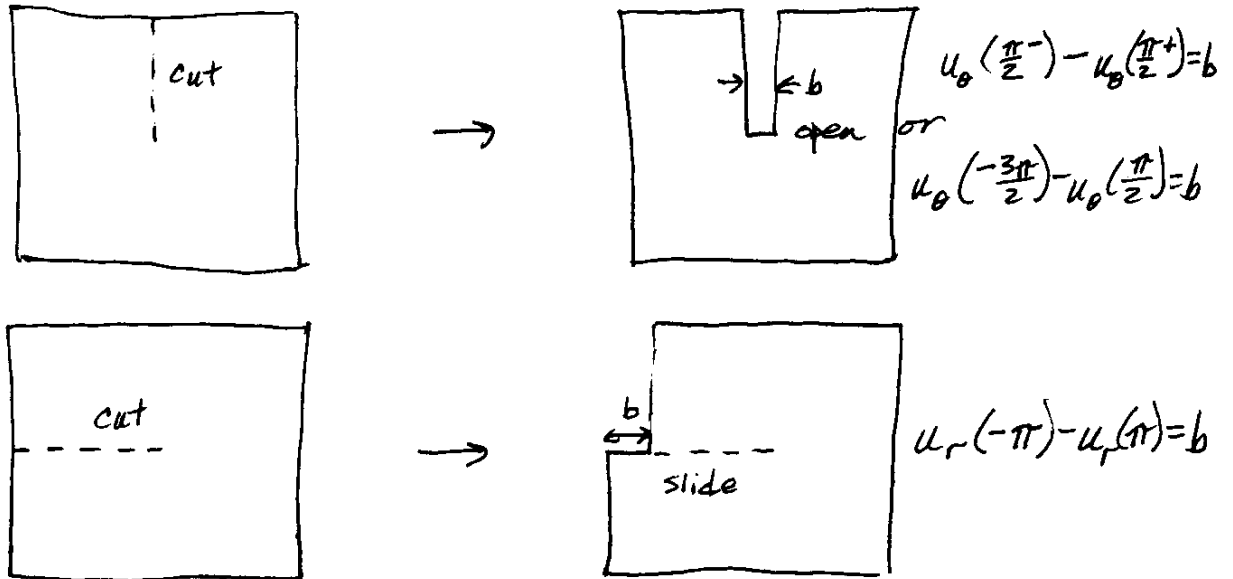
Superposition of many small point loads.

9/20/01 Dislocation (Edge)

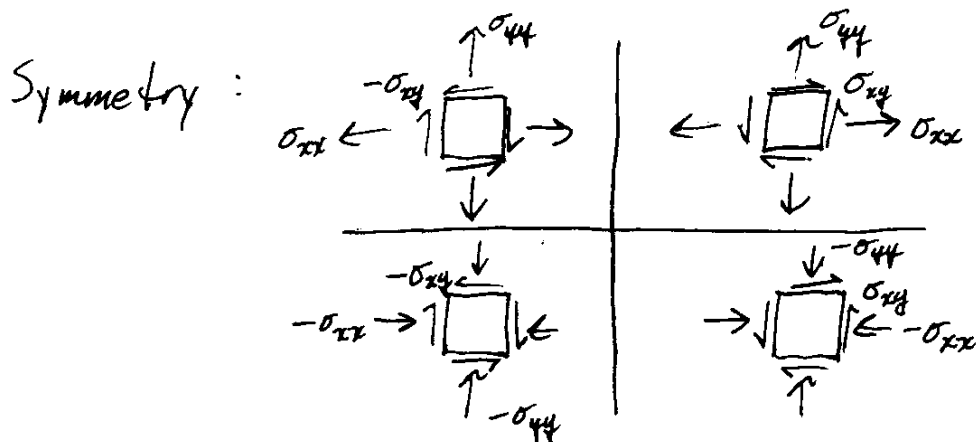


9/20/01

2 ways to create an elastic dislocation



Stresses in either case are the same but displacement fields differ.



We want  $\sigma_{rr}$  with odd functions of  $\theta$  and a displacement jump at  $\theta = \theta_0$  and  $\theta = \theta_0 + 2\pi$ .

$$\phi = A r \ln r \sin \theta + B r \theta \cos \theta$$

$$\therefore 2\mu u_r = \frac{A}{2} [-(x+1)\theta \cos\theta + (x-1)\ln r \sin\theta - \sin\theta] \\ + \frac{B}{2} [(x-1)\theta \cos\theta - (x+1)\ln r \sin\theta + \sin\theta] + PBD$$

$$2\mu u_\theta = \frac{A}{2} [(x+1)\theta \sin\theta + (x-1)\ln r \cos\theta + \cos\theta] \\ + \frac{B}{2} [-(x-1)\theta \sin\theta - (x+1)\ln r \cos\theta - \cos\theta] + PBD$$

$$2\mu u_r(-\pi) - 2\mu u_r(\pi) = \frac{A}{2} [-2\pi(x+1)] + \frac{B}{2} [2\pi(x-1)]$$

$$2\mu u_\theta(-\frac{3\pi}{2}) - 2\mu u_\theta(\frac{\pi}{2}) = \frac{A}{2} [-2\pi(x+1)] + \frac{B}{2} [2\pi(x-1)]$$

$$\therefore \pi x(B-A) - \pi(A+B) = \cancel{2\pi b} 2\mu b$$

$$\sigma_{r\theta}(\pi) = \frac{+A}{r}, \quad \sigma_{\theta\theta}(\frac{\pi}{2}) = \frac{A}{r}$$

No net force within any arc  $\rightarrow$

$$\int_{-\pi}^{\pi} (\cancel{A+B}) \cos^2\theta - \cancel{A} \sin^2\theta d\theta = 0$$

$$\therefore 2\pi B = 0 \rightarrow B = 0$$

$$\therefore -\pi A(x+1) = \cancel{b} \frac{2\mu}{\pi(x+1)} \rightarrow A = \frac{-b 2\mu}{\pi(x+1)}$$

$$\therefore \sigma_{rr} = \frac{-b\mu}{2\pi(1-\nu)} \frac{\sin\theta}{r}, \quad \sigma_{xx} = \frac{-\mu b}{2\pi(1-\nu)} \frac{y(3x^2+y^2)}{(x^2+y^2)^2}$$

$$\sigma_{\theta\theta} = \frac{-b\mu}{2\pi(1-\nu)} \frac{\sin\theta}{r}, \quad \sigma_{yy} = \frac{\mu b}{2\pi(1-\nu)} \frac{y(x^2-y^2)}{(x^2+y^2)^2}$$

$$\sigma_{r\theta} = \frac{b\mu}{2\pi(1-\nu)} \frac{\cos\theta}{r}, \quad \sigma_{xy} = \frac{\mu b}{2\pi(1-\nu)} \frac{x(x^2-y^2)}{(x^2+y^2)^2}$$

## Energy of the Dislocation

$$u = \frac{1}{2} \sigma_{ij} \epsilon_{ij}$$

$$\left. \begin{aligned} \sigma_{rr} = \sigma_{\theta\theta} &= \frac{-\mu b}{2\pi(1-\nu)} \frac{\sin\theta}{r} \\ \sigma_{zz} &= \nu(\sigma_{rr} + \sigma_{\theta\theta}) \end{aligned} \right\} \begin{aligned} \epsilon_{rr} = \epsilon_{\theta\theta} &= \frac{(1+\nu)(1-2\nu)}{E} \sigma_{rr} \\ \epsilon_{rr} = \epsilon_{\theta\theta} &= \frac{1-2\nu}{2\mu} \sigma_{rr} \end{aligned}$$

$$\therefore u = \frac{1}{2} \sigma_{rr} \epsilon_{rr} + \frac{1}{2} \sigma_{\theta\theta} \epsilon_{\theta\theta} + \frac{1}{2} \sigma_{r\theta} \epsilon_{r\theta} + \frac{1}{2} \sigma_{\theta r} \epsilon_{\theta r} = \sigma_{rr} \epsilon_{rr} + \sigma_{r\theta} \epsilon_{r\theta}$$

$$\therefore u = \frac{\mu b^2 (1-2\nu)}{8\pi^2 (1-\nu)^2} \frac{\sin^2\theta}{r^2} + \frac{b^2 \mu}{8\pi^2 (1-\nu)^2} \frac{\cos^2\theta}{r^2}$$

$$\begin{aligned} U &= \int_0^\infty \int_{-\pi}^\pi \frac{\mu b^2}{8\pi^2 (1-\nu)^2} \left( \frac{1}{r^2} - \frac{2\nu \sin^2\theta}{r^2} \right) r dr d\theta \\ &= \frac{\mu b^2}{8\pi^2 (1-\nu)^2} [2\pi \ln r - 2\nu \pi \ln r]_0^\infty \end{aligned}$$

$$U = \frac{\mu b^2}{4\pi(1-\nu)} \ln\left(\frac{R}{r_0}\right)$$

## Another Approach

$$\begin{aligned} U &= -\frac{1}{2} \int_0^\infty \sigma_{\theta\theta}(\theta = \frac{\pi}{2}) b dr = -\frac{1}{2} \int_{-\infty}^\infty \sigma_{r\theta}(\theta = \pi) b dr \\ &= \frac{\mu b^2}{4\pi(1-\nu)} \int_0^\infty \frac{1}{r} dr = \frac{\mu b^2}{4\pi(1-\nu)} \int_0^\infty \frac{1}{r} dr \\ U &= \frac{\mu b^2}{4\pi(1-\nu)} \ln\left(\frac{R}{r_0}\right) = \frac{\mu b^2}{4\pi(1-\nu)} \ln\left(\frac{R}{r_0}\right) \end{aligned}$$

## Force on a dislocation

$$F_i = -\frac{\partial W}{\partial x_i} \quad \text{where } W = \text{strain energy} + \text{potential energy of loads}$$

$W$  is an energy per length &  $F$  is a force per length. For straight dislocations

$$F_i = \epsilon_{ijk} b_p \sigma_{pj} s_k$$

$\vec{b}$  is the Burgers vector

$\vec{s}$  is a vector in the direction of the dislocation

$\underline{\underline{\sigma}}$  is the stress that would exist at the core if the  $\perp$  were not present

For the edge dislocation  $b_p = b \delta_{p1}$ ,  $s_k = \delta_{k3}$

$$F_i = b \epsilon_{ijk} \delta_{p1} \sigma_{pj} \delta_{k3} = b \epsilon_{ij3} \sigma_{ij}$$

$$\therefore F_1 = b \sigma_{12} \leftarrow \text{glide force}$$

$$F_2 = -b \sigma_{11} \leftarrow \text{climb force}$$

$$F_3 = 0$$